

# The Role of Steel Fiber of Influence on Properties of Self Compacting Concrete

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**Abstract**— Self compacting concrete (SCC) is compacting itself alone due to its self-weight and is filled almost completely while flowing in the formwork. In structural members with high percentage of reinforcement, it fills also completely all voids and gaps.

The purpose of presented research is investigating, the fresh properties of Steel Fiber SCC and the hardened properties were investigated. Fresh properties comprising flow ability, passing ability, and viscosity related segregation resistance. Hardened properties comprising compressive strength, splitting tensile strength, flexural strength, modulus of elasticity, and Ultrasonic pulse velocity.

The results indicated of the fresh properties of SCC with steel fiber, reduction in workability with increase of steel fiber content. Also the Steel fibers had effect on compressive & tensile strength, modulus of elasticity and ultrasonic pulse velocity of steel fiber self compacting concrete, there was an optimum content of steel fiber at which higher performance obtained at the both mentioned characteristics, the content was (0.75- 1)% . All fiber mixes demonstrated higher splitting tensile strength, and flexural strength relative to plain mix at all curing ages. The strengths increased as the fiber content increased. The fibers slightly decrease the U.P.V followed the same behavior as in compressive strength of SCC.

**Index Terms**— Steel fiber, Compressive strength, tensile strength, flexural strength, modulus of elasticity, Ultrasonic pulse velocity, workability.

## 1 INTRODUCTION

The Self compacting concrete (SCC) is a concrete which has the ability to flow by its own weight and achieve good compaction without external vibration. In addition, SCC has good resistance to segregation and bleeding because of its cohesive properties [1]. The raw material selection is an important part of the mix design process for SCC, since it will significantly influence the stability as well as the cost of the mix, which are two key elements in the successful use of SCC.

There is no standard method for SCC mix design, but many academic institutions, admixture ready-mix, precast and contracting companies have developed their own mix proportioning methods. Mix designs often use volume as a key parameter because of the importance of the need to over fill the voids between the aggregate particles. Some methods try to fit available constituents to an optimized grading envelope (European Project Group, 2005) [2]. Mix designs of SCC must satisfy the criteria on filling ability, passing ability and segregation resistance. The most common method of mix design is the general method developed by the University of Tokyo and since then, many attempts have been made to modify this method to suit local conditions [3]. In general, the following rules should be followed to be successful in manufacturing SCC [4], with certain methods are used, there are many methods for design of mixes such as Rational methods [5], and EFNARC methods [2].

SCC is known to be brittle and can easily crack under low levels of tensile force. Inherently Plain, unreinforced concrete is a brittle material, with a low tensile load and a low strain capacity. This behavior of brittleness can be overcome by using randomly oriented short discrete fibers. Fibers not only suppress the formation of cracks, but also abate their propagation and growth. Fibers have been added to hardened state

Fiber reinforced concrete becomes an option whenever durability (limited crack widths) or safety considerations are design criteria. They improve the performance (strength and toughness) of brittle cement-based materials by bridging cracks, transmitting stress across a crack and counteracting the crack growth [6]. The steel fiber is the most common fiber type in the building industry; plastic, glass and carbon fibers contribute to a smaller part to the market [7]. There are various types of steel fibers such as wave cut, end large steel fiber, deformed sheet and also hooked end steel fiber. In general, hooked end steel fibers are widely used in the fiber reinforced concrete because it has higher strengthening effect on the cement matrix as compared with other types of steel fibers [8]. Merging steel fibers with SCC to produce steel fiber reinforced self-compacting concrete (SFSCC) is, therefore, highly desirable and carries a lot of potential for the concrete industry. SFSCC is an innovative type of concrete, which combines the advantages and extends the possibilities of both SCC and steel fiber reinforced concrete, dependent on the type and the content of the fibers, the workability of SCC can be significantly affected [9].

In conventional concrete, Steel fibers have been applied to replace bar reinforcement, to decrease the width of cracks, to increase tensile and flexural strength, and to improve the post-cracking behavior. Steel fiber reinforcement influences the way cracks develop in concrete and may impart improved crack growth resistance, increased surface roughness of individual cracks, and a greater likelihood for crack branching and multiple crack development. Due to this, steel fiber reinforcement may be used to significantly reduce the permeability of concrete, thus improve the durability [10].

With regard to workability, the main parameters affecting it are the fiber shape, the aspect ratio, the fiber length and the volume of fibers per m<sup>3</sup> of concrete. Generally the same parameters which decrease the workability conversely increase the performance. Therefore it is important to find a compromise between

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**WORKABILITY AND PERFORMANCE [11]** . Steel Fibers need to be homogeneously distributed and clustering of fibers must be counteracted in order to optimize the performance of the fiber. The critical fiber content is surpassed when a stiff structure of the granular skeleton makes flow under concretes' own weight impossible [12] [13].

In principle, the properties of the fresh self-compacting concretes should not be different from conventional concrete. Only one exception is regarding the consistency. SCC is generally distinguished by their special fresh concrete properties[14] .The workability of SCC is higher than the highest class of consistence described for conventional concrete and can be characterized by Filling ability, Passing ability and Segregation resistance ( European Project Group, 2005) [2].

Fibers affect hardened properties in different rates ,fibers have little effect on compressive strength , a moderate on splitting tensile strength but they are generally found to have much greater effect on the flexural strength of SFRC.

Steel fibers do not have the significant influence on the compressive behavior of concrete due to the small volume of fibers in concrete mix[15].

(Maidl, 1995[16]; König & Kützing, 1999[17]; Sato et al., 2000[18]) found that the addition of the steel fibers has hardly any effect on the compressive strength just as on the tensile strength. The ductility increases due to the addition of the fibers.

In Iraq , almost searcher are used lime stone powder as a powder in SCC mixes with cement , the lime stone powder is inactive material [19],similar activated of aggregate but it has finer than the fine aggregate .The used of this materials may be change of the properties of SCC. From this point is carried out this study to evaluate of performance of SCC with different steel fiber content at fresh and harden states .

For a newly developing material like self compacting concrete reinforced by steel fiber, studies on durability are of paramount importance for instilling confidence among the engineers and builders.

## 2 EXPERIMENTAL WORKS

### 2.1 Materials used

#### 2.1.1 Cement

Ordinary Portland cement, which has specific gravity of 3.15 and the SO<sub>3</sub> of 2.6, was used in this investigation. It is conforming to IQS:5 -1984[ 20].

#### 2.1.2 Coarse aggregate

Rounded shape aggregate of size of 10 mm was used and it has the following properties: Specific gravity of 2.6 and the SO<sub>3</sub> of 0.07%.

#### 2.1.3 Fine aggregate

Natural sand conforming to zone III of IQS: 45 - 1984[21] was used and its properties are found as follows: Specific gravity 2.57 and the SO<sub>3</sub> of 0.4%.

#### 2.1.4 Water & Super-plasticizer

The drinking tap water has been used for both mixing and curing of concrete. A chemical admixture based on modified polycarboxylic ether, which is known commercially (Glenium 51) was used in producing SCC as a superplasticizer admixture. It was complied with (IQS No.1431-89)[22] and (ASTM C494-05)[23] type F .

#### 2.1.5. Lime stone powder (LSP)

This material was used to increase the amount of powder (cement + filler). It has SO<sub>3</sub> of 1.9 and its specific gravity was 2.8.

#### 2.1.6. Fibers

In this work, type of steel fiber having geometry of cylindrical with hooked ends was used. The characteristics of the steel fiber ; length, diameter ,tensile strength, specific gravity were 30mm , 0.5 mm,1100 MPa and 7000 kg/m<sup>3</sup> respectively. It was complied with (ASTM A820M-11)[24].

### 2.2.Methodology

The mix design method used in the present study was according to( EFNARC 2005) [2] . Table (1) shows the proportions of reference plain mixture.

TABLE 1  
PROPORTIONS OF REFERENCE PLAIN MIXTURE

Cement kg/m <sup>3</sup>	Sand kg/m <sup>3</sup>	Gravel kg/m <sup>3</sup>	LSP kg/m <sup>3</sup>	SP L/m <sup>3</sup>	Water L/m <sup>3</sup>	w/c	w/p
430	840	675	130	4.5	210	0.49	0.32

Fibers were added in quantities ranging from 0 to 1.5 % by volume of total mixture. Fibers were fed into mixer by hand to ensure that clumping and clustering effects were minimized. Fiber content in FRSCC mixtures are detailed in Table (2):

TABLE 2  
FIBER CONTENT IN FRSCC MIXTURE

Mix Symbol	Fiber Content by (Vol.%)
R	Reference mix without fiber
ST1	0.5 % steel hooked end hard-drawn wire fiber
ST2	0.75 % steel hooked end hard-drawn wire fiber
ST3	1.0 % steel hooked end hard-drawn wire fiber
ST4	1.25 % steel hooked end hard-drawn wire fiber
ST5	1.5 % steel hooked end hard-drawn wire fiber

The mixing process was performed in a drum laboratory mixer of 0.05 m<sup>3</sup> to mix concrete ingredients. For SCC mixes which require no compaction work, the mixes being poured into the tight steel molds until its fully filled without any compaction. All specimens were demoulded after 24 hours and initial curing in tap water was performed. The curing time was for three ages (7, 28 and 90) days, until the specimens are tested .

### 2.3. Fresh Concrete Tests

The mix design should be tested by more than one method for the different workability parameters. Slump flow, T500, L-box and V-funnel test were used for assessment of fresh properties of SCC in this study.

The fresh properties of plain SCC and SCC reinforced by steel fiber were tested by the procedures of (European Guidelines for self compacting concrete) [2].

### 2.4. Hardened Concrete Tests

The mechanical properties studied are compressive strength, splitting tensile strength, flexural strength and static modulus of elasticity. Furthermore, the non-destructive test methods, Ultra-sonic pulse velocity test and Schmidt hammer test are used. The compressive strength test was performed in accordance with IQS:348-1992 [25] using 150 mm cube specimens. The splitting tensile strength test was carried out according to IQS:283-1995 [26] using  $\text{Ø}100 \times 200$  mm cylinder specimens. The test procedure given in IQS:291-1991 [27] was used to determine the flexural strength using  $100 \times 100 \times 400$  mm prisms. The static modulus of elasticity was performed according to IQS:370-1993 [28] by using test cylinders of  $\text{Ø}150 \times 300$  mm. The Ultrasonic Pulse Velocity test was conducted according to IQS:300-1993 [29] by using 150 mm cube specimens.

## 3. RESULTS AND DISCUSSIONS

### 3.1 The Effect of steel fiber on the fresh Properties of Self-Compacting concrete

#### 3.1.1 Slump Flow and T500 mm

The flowing ability of fresh concrete is described by slump flow investigated with Abrams cone. Table (3) and Fig. (1) show the results of slump flow tests. The values of (D) represent the maximum spread (slump flow final diameter). All the mixtures had a slump flow diameter between (550-750) mm. Significant decrease in slump flow diameter has been observed with incorporating steel fibers in SCC mixes, adding steel fibers increases the resistance to flow and reduces the flowability due to increasing the interlocking and friction between fibers and aggregate [30]. The decrease in slump flow was found to be increase with the increase in steel fiber content. This is likely due to the effect of the higher number of steel fibers as well as higher internal resistance of the steel fibers in fresh concrete mixtures [31]

TABLE 3  
RESULTS OF FRESH CONCRETE TESTS

Mix	Steel Fiber (% by Vol.)	D (mm)	T <sub>500</sub> (Sec)	Blocking Ratio (BR)	T <sub>v</sub> (sec)
SF1	0	750	2.1	0.97	6.6
SF2	0.5	720	2.28	0.94	7.0
SF3	0.75	680	2.95	0.90	7.7
SF4	1	670	3.14	0.85	8.3
SF5	1.25	625	3.65	0.82	8.8
SF6	1.5	584	4.25	0.8	9.3

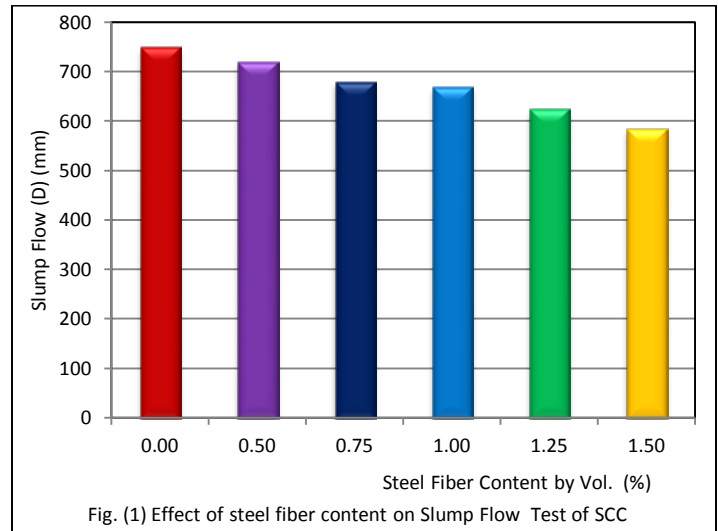


Fig. (1) Effect of steel fiber content on Slump Flow Test of SCC

While the values of T500 mm represent the time required to the concrete flow to reach the 500mm diameter during the slump flow test was also measured and recorded as T500 (sec) (Slump Flow Time), which indicates the speed of flow and hence the viscosity of the self-compacting concrete. Table (3) and Fig. (2) show the results of T500mm. The values were ranged between (2.1-4.25) sec. According to the (European Project Group, 2005) [2], SCC can be classified as VS1 for T500  $\leq 2$  sec or VS2 for T500  $> 2$  sec.

Including steel fiber in SCC mixtures resulted in an increase in T500 values. The increase percentages found to be increase with the increase in fiber content. The increasing of slump time due to increase of internal friction resultant from higher number of steel fiber.

All results of slump flow and slump flow time were conformed EFNARC 2005 limitations.

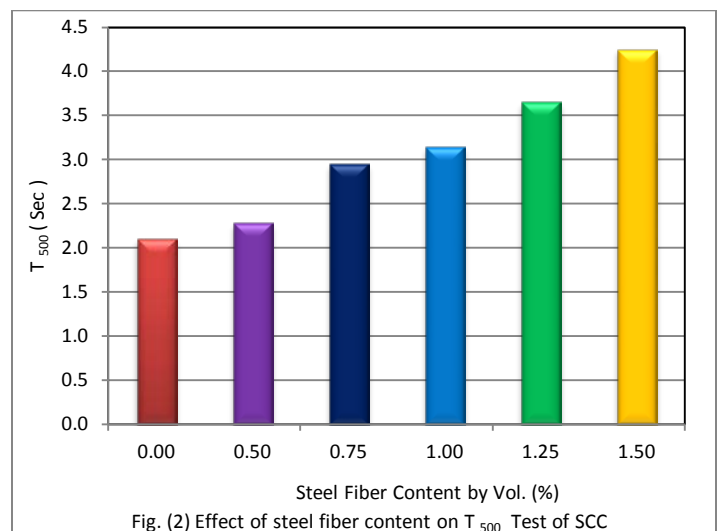


Fig. (2) Effect of steel fiber content on T<sub>500</sub> Test of SCC

#### 3.1.2 L-box passing ability

The L-Box with 2 bars was used in this study to assess the passing ability of the mixes. The Blocking Ratios results

(BR=H2/H1) of the tests are summarized in Table (3) & plotted in Fig. (3). The results of the BR ranged between (0.8-0.97). The results show that the BR decreased with increasing steel fiber content . The higher the steel fiber content, the lower the BR was.

Introducing steel fibers had its impact on passing ability. The decreased the BR with increasing steel fiber content due to internal resistance of flow of concrete result of increasing of number of steel fiber [32] .

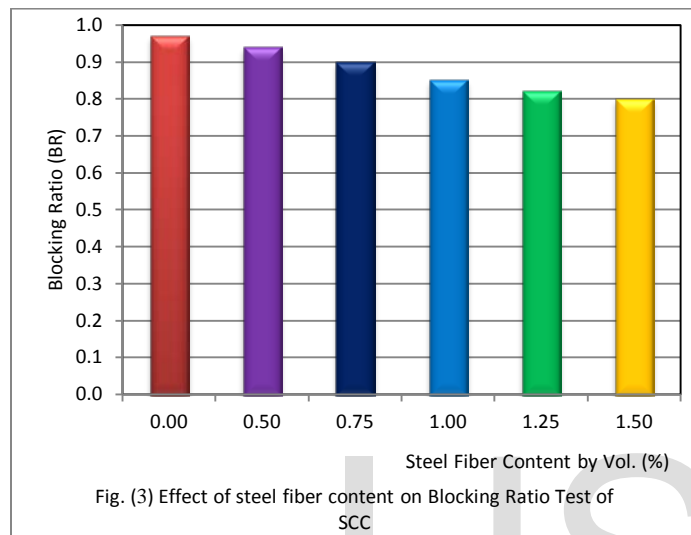


Fig. (3) Effect of steel fiber content on Blocking Ratio Test of SCC

### 3.1.3.V-Funnel Flow Time

The V-funnel test is used to assess the viscosity and filling ability of self-compacting concrete [2] . Table (3) shows the results of V-funnel test. The  $T_v$  values ranged between(6.6-9.3) sec. SCC is classified into two categories: VF1 ( $\leq 8$  sec) and VF2 (9-25) sec (European Project Group, 2005).

The results fall in VF1 and VF2 categories, referring to that the mixes have moderate to relatively high viscosity , indicating reduced filling ability and at the same time ameliorated segregation resistance.

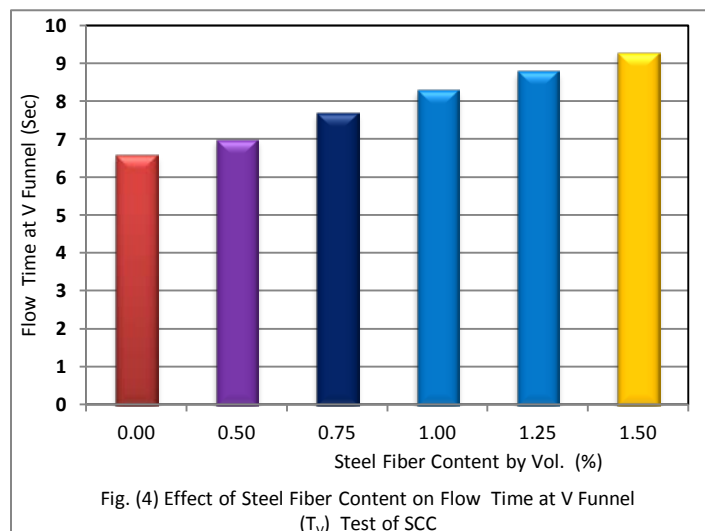


Fig. (4) Effect of Steel Fiber Content on Flow Time at V Funnel ( $T_v$ ) Test of SCC

V-Funnel flow time increased by incorporating steel fibers in mixes as illustrated in Fig.(4). Similar behavior was observed in the T500 test. Besides, the higher the steel fiber content, the more the flow-time increased. This can be ascribed to, the increasing in fiber content leads to increase the friction between the fibers and aggregates and the friction of the fibers with each other which could extend the required time to empty the V-funnel [33].

## 3.2.The effect of steel fiber on hardened properties of SCC

### 3.2.1.Compressive strength

The compressive strength, is one of the most important properties of hardened concrete .Table (4) and Fig.(5) show the average of the results of The compressive strength test at 7, 28and 90 days. The results indicate that all specimens exhibited a continuous increase in compressive strength with progress in age. This increase in compressive strength with age is due to the continuity of hydration process which forms a new hydration product within the concrete mass [34].Also the results indicate to compressive strength of the SCCs increases by steel fiber introduction, the compressive strength increases as the steel fiber volume increases up to 0.75% beyond, the compressive strength decreases ,however, it still higher than the plain SCC as illustrated in Fig.(6) .The percentage of changes (increase ) in compressive strength for all mixes with steel fiber content are represented in Table (4).

The increase in compressive strength may be associated with uniform dispersion of fine fibers throughout self-compacting concrete of high flowability, leading to consistent internal integrity. Also this improvement in the compressive strengths of the steel fiber reinforced SCC refer to the control of cracking and the mode of failure by means of post cracking ductility . However, steel fibers caused crack closing forces which led to increase the compressive strength. While, fiber addition caused the increase of entrapped air which decreased the compressive strength. Both effects in combination had the tendency to a little increase in compressive strength especially in SCC mixes reinforced with fibers higher than 0.75%. therefore, the steel fibers could increase the compressive strength only if it does not change, to a certain limit, the air content [30]

TABLE 4  
RESULTS OF COMPRESSIVE STRENGTH TEST OF SCC

Mix	Compressive Strength(MPa)			Change in Compressive strength with respect to reference mix (SF1) %		
	7 days	28days	90days	7 days	28 days	90 days
SF1	27.5	35.4	45.1	-	-	-
SF2	30.4	37.6	47.2	10.55	6.21	4.66
SF3	35.6	45.2	48.3	29.45	27.68	7.10
SF4	34.5	43.2	47.2	25.45	22.03	4.66
SF5	33.3	42.5	46.5	21.09	20.06	3.10
SF6	33.1	42.1	46.0	20.36	18.93	2.00

strength due to inclusion of fibers [7] [35].

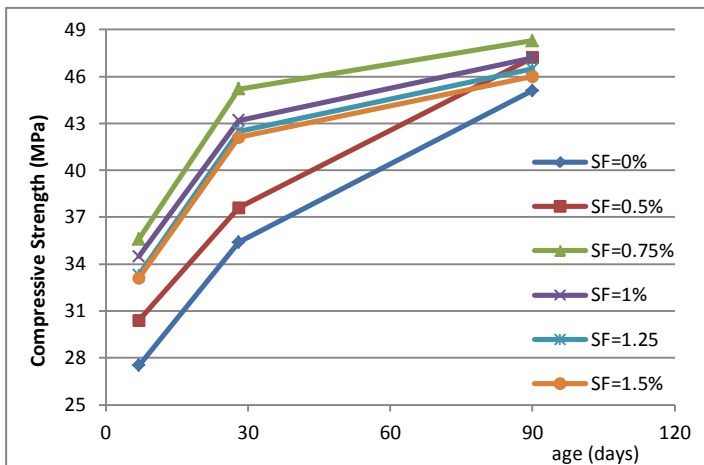


Fig.(5) Development of compressive strength with age for different SF content in SCC

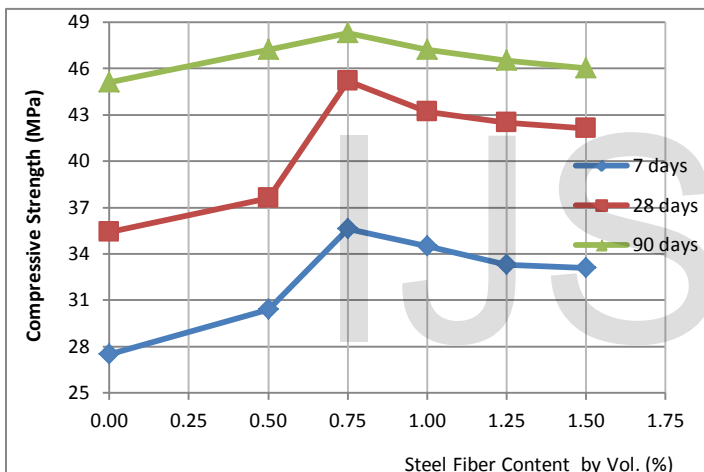


Fig.(6) Effect of SF content on compressive strength in SCC at age (7,28 and 90) days

**TABLE 5**  
**RESULTS OF SPITTING TENSILE STRENGTH TEST OF SCC**

Mix	Spitting Tensile Strength (MPa)			Change in Spitting Tensile strength with respect to reference mix (SF1) %		
	7 days	28 days	90 days	7 days	28 days	90 days
SF1	3.0	3.7	4.25	-	-	-
SF2	3.7	4.3	5	23.33	16.22	17.65
SF3	4.6	5.0	5.5	53.33	35.14	29.41
SF4	4.7	5.3	6.0	56.67	43.24	41.18
SF5	4.9	5.4	6.5	63.33	45.95	52.94
SF6	5.0	5.5	6.5	66.67	48.65	52.94

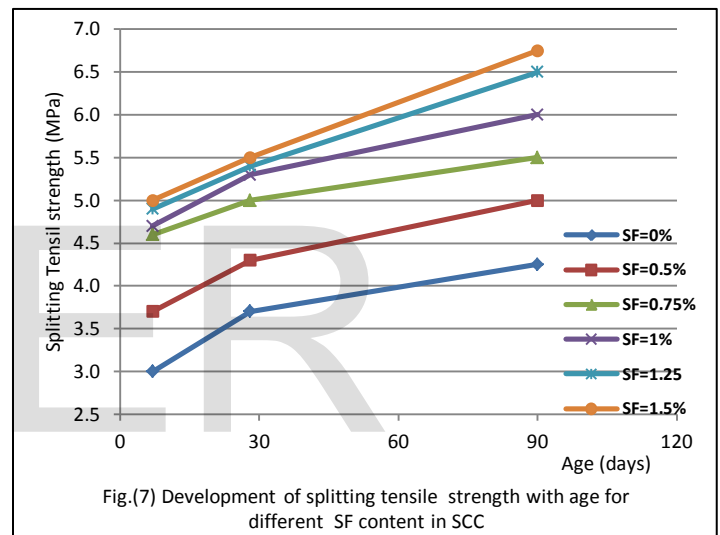


Fig.(7) Development of spitting tensile strength with age for different SF content in SCC

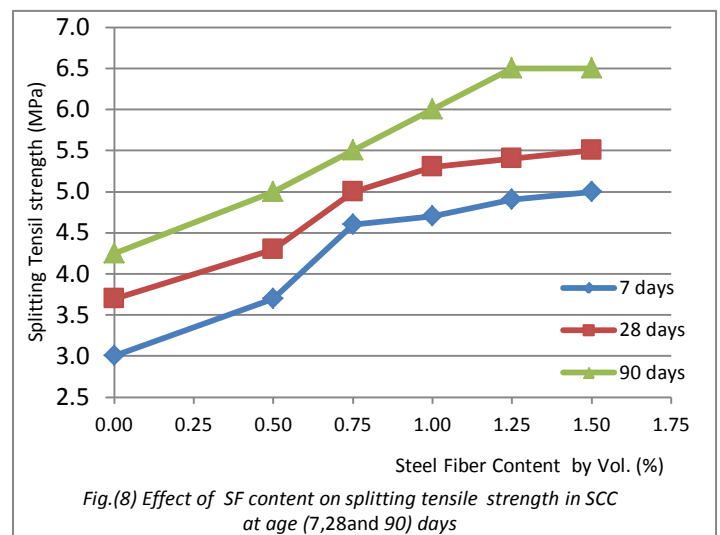


Fig.(8) Effect of SF content on spitting tensile strength in SCC at age (7,28 and 90) days

### 3.2.2 Splitting Tensile strength

The results splitting tensile strength indicate that all specimens exhibited a continuous increase in splitting tensile strength with progress in age as summarized in Table (5) and Fig.(7). Also, the results observed that splitting tensile strength of the SCCs increases with steel fiber content. The splitting tensile strength increases with increasing in steel fiber volume up to 1.5%. Also the percentage of increase in splitting tensile strength are listed in Table (5) and illustrated in Fig (8). The strength increase due to inclusion of steel fibers is attributed to the mechanism of steel fibers in arresting crack progression. Where, the presence of fibers in concrete restrains the development of internal microcracks and thus contributes to an increased tensile strength. Accordingly, the increase in fiber content leads to an increase in the tensile strength of concrete. Moreover, the improved bonding of fiber-matrix that provided by the shape of the steel fiber, hooked ends, boosted pullout strength and thus, yielded high increment in

### 3.2.3 Flexural strength

Similar to splitting tensile strengths of concrete, the results indicate that all specimens exhibited a continuous increase in flexural strength with progress in age as shown in Table (6) and Fig.(9). The results indicate to effect of steel fiber on flexural tensile strength of the SCCs was very clearly, and they showed the benefit of steel fibers to improve of flexural strength, This improvement in flexural strength increases with the increase in steel fiber volume up to 1.5%

The flexural strength indicated significant increase in strength due to the inclusion of steel fibers. The percentages of increase in flexural tensile strength for all mixes are summarized in Table (6) and illustrated in Fig.(10).

The flexural strength increases, due to the superior performance in flexural strength for specimens with steel fibers arises from the improved fiber - matrix bond provided by using steel fiber with hooked ends [36].

Also, the percent of increase in flexural strength was found to be increased with the increase in steel fiber content. This behavior is mainly attributed to the role of steel fibers in releasing fracture energy around crack tips which is required to extent crack growing by transferring it from one side to another side [37].

When the tensile stress is transferred to fibers, the transfer can arrest the propagating macro-cracks and substantially improve. Crack control plays a crucial role in the performance of concrete in service. The loads may overstress hardened concrete from cracking, leading from cracking to substantial failure in concrete. Thus, incorporation of discrete fibers in vulnerable concrete is useful and effective. The resulting fiber-reinforced concrete exhibits satisfactory resistance to crack formation and propagation[7].

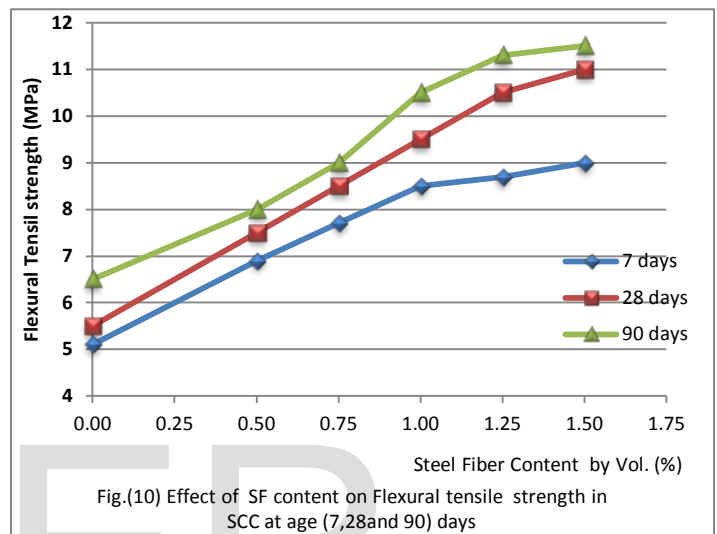


Fig.(10) Effect of SF content on Flexural tensile strength in SCC at age (7,28and 90) days

TABLE 6

RESULTS OF FLEXURAL TENSILE STRENGTH TEST OF SCC

Mix	Flexural Tensile Strength Test (MPa)			Change in Flexural Tensile strength with respect to reference mix (SF1) %		
	7 days	28 days	90 days	7 days	28 days	90 days
	SF1	5.1	5.5	6.5	-	-
SF2	6.9	7.5	8	35.29	36.36	23.08
SF3	7.7	8.5	9	50.98	54.55	38.46
SF4	8.5	9.5	10.5	66.67	72.73	61.54
SF5	8.7	10.5	11.3	70.59	90.91	73.85
SF6	9	11	11.5	76.47	100.00	76.92

### 3.2.4. Modulus of elasticity

The static modulus of elasticity for all mixes is experimentally determined at ages 28 and 90 days, the results of this test are listed in Table (7). The results listed in Table (6) and plotted in Fig.(11) show steel fibers demonstrated similar impact on elastic modulus as on compressive strength. However, the increments due to incorporating steel fibers were insignificant, the percentages of change in elastic modulus for SCCs are listed in Table (7) and plotted in Fig.(12) Since, the main role provided by steel fiber starting after the concrete suffering plastic deformation, thus, steel fiber would not contribute much to elastic deformation stage [38] [39].

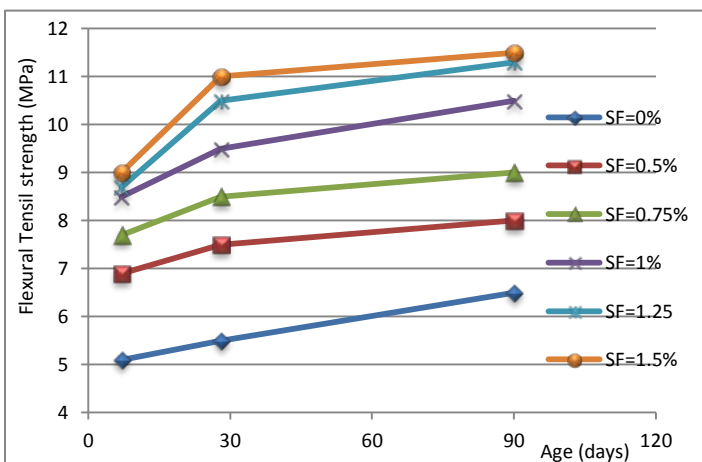
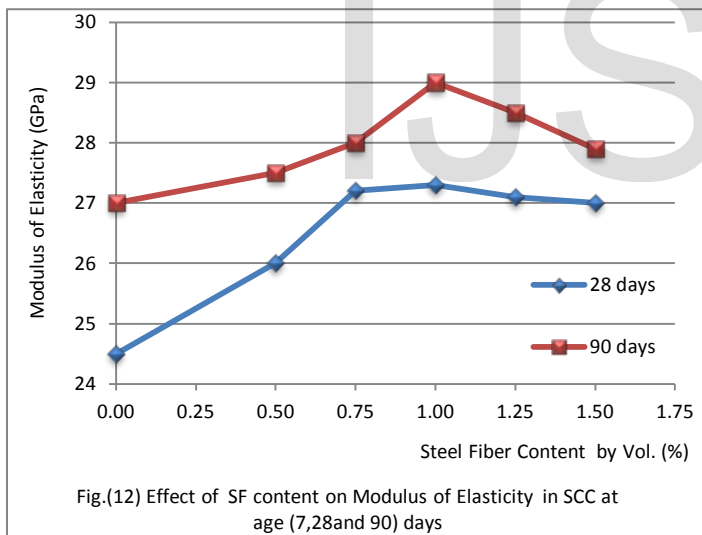
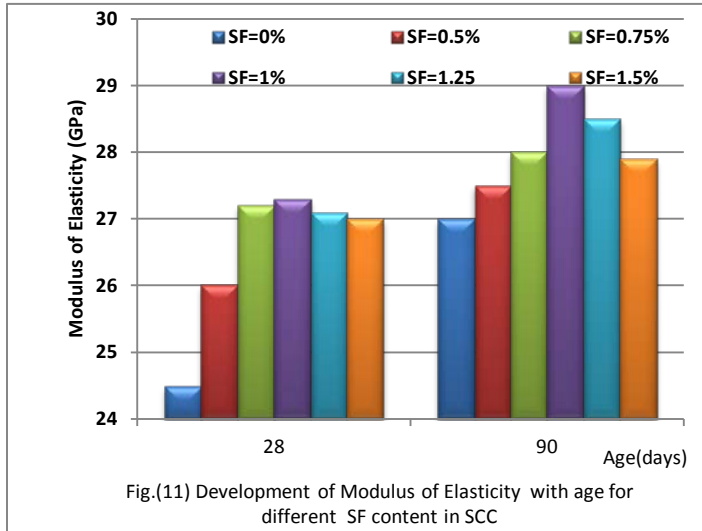


Fig.(9) Development of Flexural tensile strength with age for different SF content in SCC

TABLE 7  
 RESULTS OF MODULUS OF ELASTICITY TEST OF SCC



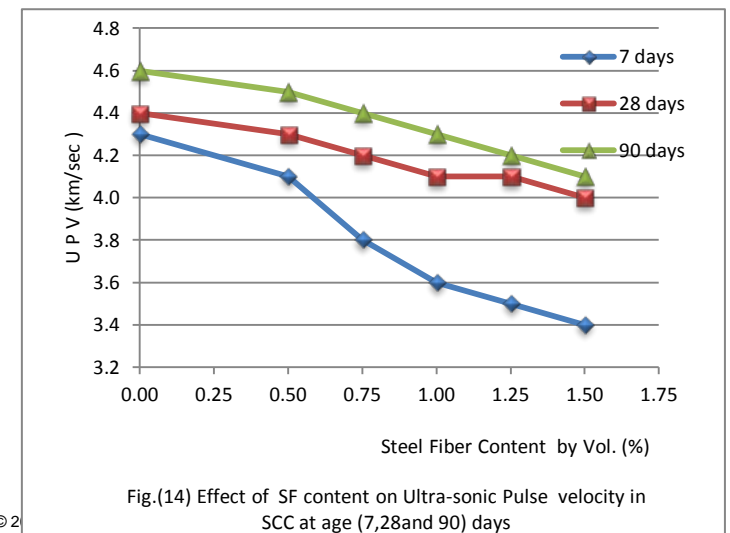
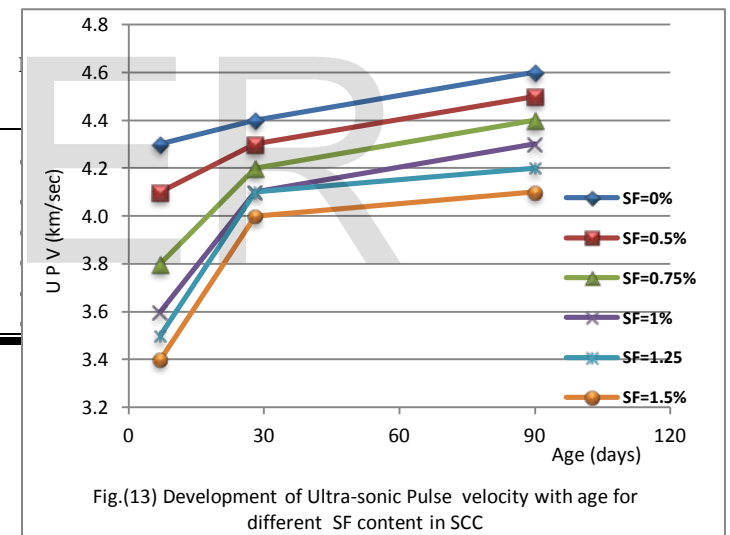
**3.2.5.Ultra-sonic Pulse velocity**

Ultrasonic pulse velocity increased with the increase in curing age of SCC mixes . The results of ultrasonic pulse velocity for the various types of SCC mixes at (7, 28 and 90 days) are presented in Table (8) and Fig.(13). It can be seen that introducing steel fibers negatively affected the ultrasonic pulse velocity as shown in Table (8) and Fig.(14). This might be attributed to the increase of the amount of entrapped air voids due to incorporation of fibers into the mixes. besides, the fibers inside cube were randomly oriented, when the wave pass through the fibers the wave maybe deflected to other directions rather than pass straight forward to the end of the cube

[35].

Mix	Modulus of Elasticity Test (GPa)		Change in Modulus of Elasticity with respect to reference mix (SF1) %	
	28 days	90 days	28 days	90 days
SF1	24.5	27	-	-
SF2	26	27.5	6.12	1.85
SF3	27.2	28	11.02	3.70
SF4	27.3	29	11.43	7.41
SF5	27.1	28.5	10.61	5.56
SF6	27	27.9	10.20	3.33

TABLE 8  
 RESULTS OF ULTRA-SONIC PULSE VELOCITY TEST OF SCC



#### 4. CONCLUSIONS

1. Overall, slump flow diameter (flowability) and L-Box blocking ratios (passing ability) decrease with the increase in steel fiber content of the concrete mixtures with respect to plain mixtures. However, all mixes are satisfied to SCC requirements .
2. Slump flow time and V-funnel flow time increase with the increase in steel fiber content of the concrete mixtures with respect to plain mixtures.
3. Steel fibers increased in compressive strength and modulus of elasticity at early age, but at age 90 days, steel fibers had a marginal increments in compressive strength and modulus of elasticity compared to the increments in the other mechanical properties.
4. All steel fiber mixes demonstrated a higher splitting tensile strength and flexural strength relative to plain mixes at all curing ages. The tensile strength increased as the fiber content increased, however, the increments in flexural strength were higher than splitting tensile strength with more than 90% increments having been recorded.
5. Ultrasonic pulse velocity decreased with including steel fiber in SCC.
6. The highest steel fiber content 1.5 (% by Vol.) had, in general, best effect on hardened properties but the worst on fresh properties of SCC.
7. As well,(0.75, 1)% steel fiber content were sufficient for achieving satisfying (optimum ) performance in fresh and hardened properties of SCC.

#### ACKNOWLEDGMENT

The authors thanks Mahoumd R.H.and Ahmed S.M. for supported him to compelet this work .

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